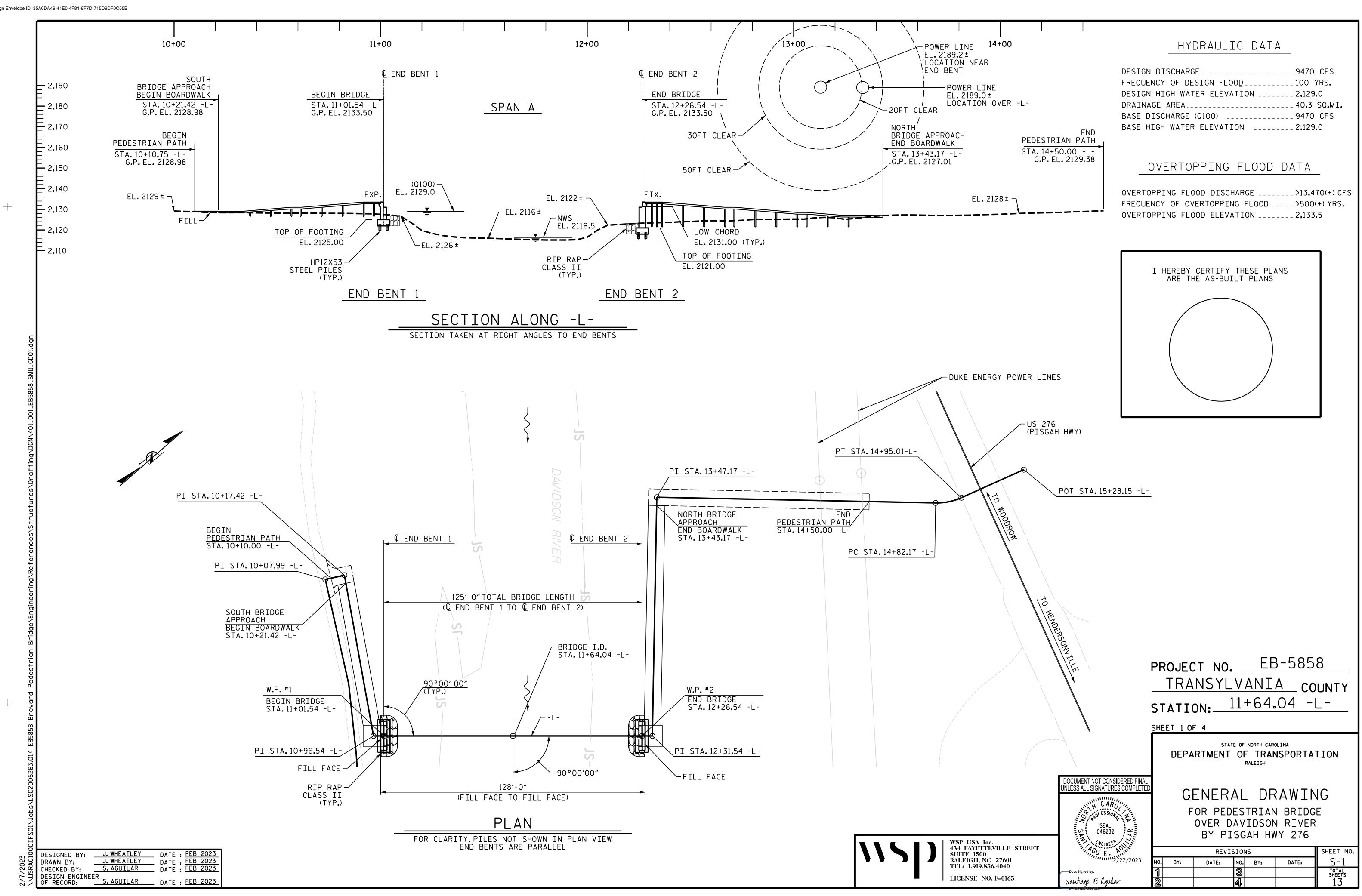
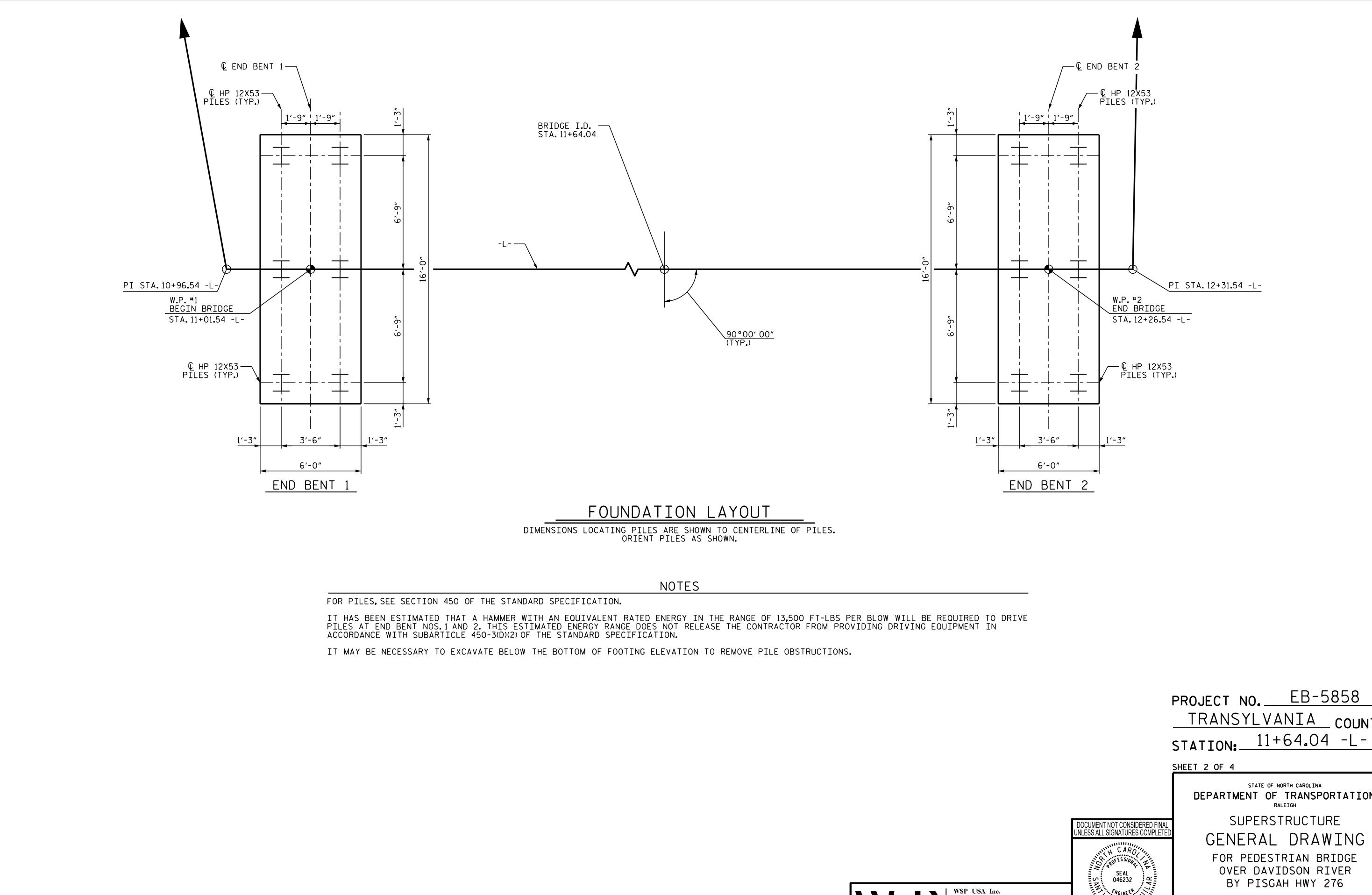
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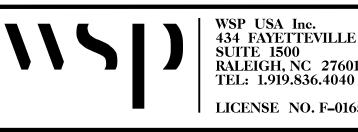


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اور ^س	DESIGNED BY:	J. WHEATLEY	DATE : FEB 2023
4G:	DRAWN BY:	J. WHEATLEY	DATE : FEB 2023
2C 5R/	CHECKED BY:	S.AGUILAR	DATE : FEB 2023
2/7/ 2///S	DESIGNED BY: _ DRAWN BY: _ CHECKED BY: _ DESIGN ENGINEEF OF RECORD: _	S. AGUILAR	DATE : <u>FEB 2023</u>



	PROJECT NO. <u>EB-5858</u> <u>TRANSYLVANIA</u> COUNTY STATION: <u>11+64.04</u> -L-
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE GENERAL DRAWING FOR PEDESTRIAN BRIDGE OVER DAVIDSON RIVER BY PISGAH HWY 276
LE STREET $GO = GO $	REVISIONS SHEET NO.
$\begin{array}{c} 601 \\ 2/27/2023 \\ \hline \\ 0165 \\ \hline \\ \\ \hline \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $	NO.BY:DATE:S-213TOTAL SHEETS
0165 Santiago E líquilar	2 4 13

SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

5.15					Driven Piles			Predrilling for Piles*			Drilled-In Piles		
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Resistance per Pile TONS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Lenth per Pile FT	Scour Critical Elevation FT	Min Pile Tip (Tip No Higher Than) Elev FT	Required Driving Resistance (RDR)** per Pile TONS	Total Pile Redrives Quantity EACH	Predrilling Length per Pile Lin FT	Predrilling Elevation (Elev Not To Predrill Below) FT	Maximum Predrilling Dia INCHES	Pile Excavation (Bottom of Hole) Elev FT	Pile Exc Not In Soil per Pile Lin FT	Pile Exc In Soil per Pile Lin FT
End Bent 1, Piles 1-6	30	2123	25	2109		55		25	2098	12			
End Bent 2, Piles 1-6	30	2119	20	2106		55		20	2098	12			

*Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length. Nominal Scour Resistance ***RDR*

 $\frac{Factored Resistance + Factored Downdrag Load + Factored Dead Load}{Factored Resistance} + Nominal Downdrag Resistance + \frac{Nominal Scour Resistance}{Scour Resistance Factored}$

PILE DESIGN INFORMATION (Blank entries indicate item is not applicable to structure)

End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile TONS	Factored Downdrag Load per Pile TONS	Factored Dead Load* per Pile TONS	Dynamic Resistance Factor	Nominal Downdrag Resistance per Pile TONS	Nominal Scour Resistance per Pile TONS	Scour Resistance Factor (Default = 1.00)
End Bent 1, Piles 1-6	30			0.60		5	1.00
End Bent 2, Piles 1-6	30			0.60		5	1.00

*Factored Dead Load is factored weight of pile above the ground line.

NOTES:

1. The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Jacob Wessell, P.E., NC PE 030395) on 10-26-2021. 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance. 3. The Engineer will determine the need for PDA Testing and Pipe Pile Plates when PDAs or plates may be required.

	Pile Driving Analyz	Pile Order Lengths			
End Bent/ Bent No	No Required? Length Quantit			End Bent/ Bent No(s)	Pile Order Length Basis* EST or PDA
			-		

*EST = Pile order lengths from estimated pile lengths; PDA = Pile order lengths based on PDA testing. For groups of end bents/bents with pile order lengths based on PDA testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the PDA.

SUMMARY OF PILE ACCESSORIES (Blank entries indicate item is not applicable to structure)

End Dont/	Dine Dile	S	teel Pile Points		Steel Pile Tips Required? YES	
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Pipe Pile Plates Required? YES or MAYBE	Pipe Pile Cutting Shoes Required? YES	Pipe Pile Conical Points Required? YES	H-Pile Points Required? YES		
End Bent 1, Piles 1-6				YES		
End Bent 2, Piles 1-6				YES		
TOTAL QTY:				12		

SIGNATURES COMPLETED

2

SUMMARY OF PDA/PILE ORDER LENGTHS (Blank entries indicate item is not applicable to structure)

		-	COUNTY					
	STA	ATION:			11+64	.04 -L-		
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Santiago E Aguila ^{2/27/2023}								
-25185NATURE DATE	REVISIONS SHEET NO. S-3							
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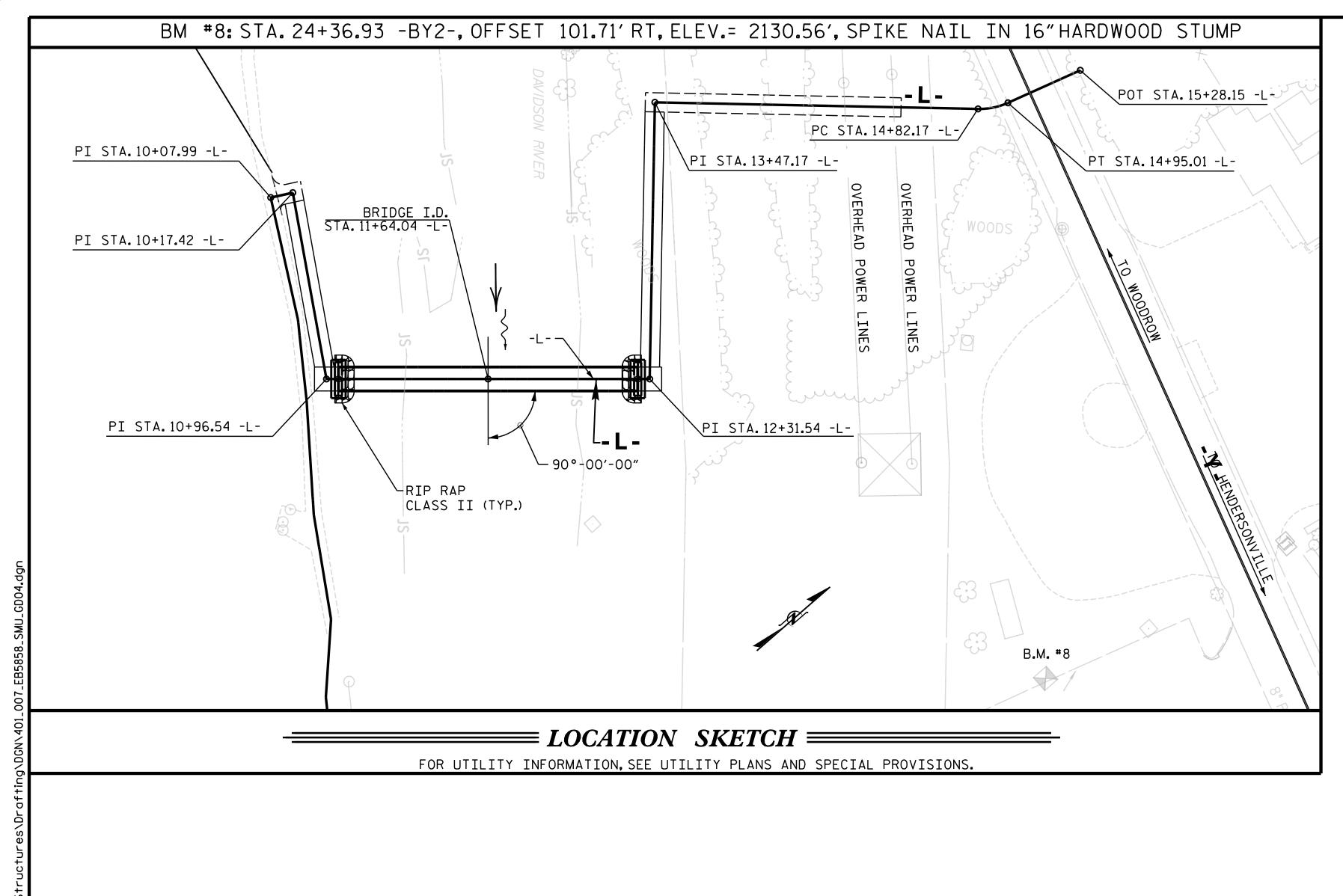
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PROJECT NO.

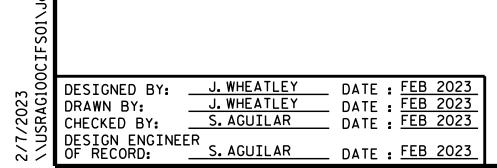
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	FOUNDATION EXCAVATION FOR END BENT	REINFORCED CONC.DECK SLAB	CLASS A CONCRETE		PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	НР	12 X 53 EEL PILES	STEEL PILE POINTS	PRE-DRILLING FOR PILES	RIP RAP CLASS II (2'-O" THICK)	GEOTEXTILE FOR DRAINAGE	* PRE- FABRICATED PEDESTRIAN BRIDGE	* BRIDGE APPROACH BOARDWALKS
	LUMP SUM	SQ.FT.	CU.YDS.	LBS.	EA.	No.	LIN.FT.	EA.	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE		1250.0										LUMP SUM	LUMP SUM
END BENT 1	LUMP SUM		22.3	2,838	6	6	150	6	150	27	12		
END BENT 2	LUMP SUM		28.6	4,129	6	6	120	6	120	27	12		
TOTAL	LUMP SUM	1250.0	50.9	6,967	12	12	270	12	270	54	24	LUMP SUM	LUMP SUM





PEDESTRIAN LIVE LOAD = 90 PSF

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND MEETS THE CRITERIA SET FORTH BY THE AASHTO GUIDE SPECIFICATIONS FOR DESIGN OF PEDESTRIAN BRIDGES.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET "STANDARD NOTES" SHEET. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

THE SCOUR CRITICAL ELEVATION FOR END BENT NO.1 IS ELEVATION 2109. AND END BENT 2 NO.2 IS ELEVATION 2106. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

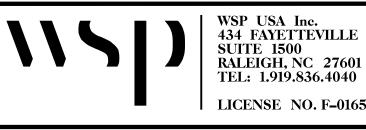
THE CLASS AA CONCRETE IN THE DECK SLAB SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE SLAB.

THE CONTRACTOR SHALL BE ADVISED THAT THE PROPOSED PEDESTRIAN BRIDGE IS NEAR OVERHEAD POWER LINES. POWER LINE LOCATIONS AND CLEARANCES SHOWN ON THE PLANS ARE FOR INFORMATION ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING ALL UTILITY COMPANIES AND SURVEY THE LOCATION OF UTILITIES PRIOR TO COMMENCEMENT OF THE WORK. THE CONTRACTOR SHALL BE ADVISED THAT OVERHEAD POWER LINES ARE SUBJECT TO MINIMUM TEMPORARY AND PERMANENT CLEARANCE ENVELOPES AS ESTABLISHED BY THE POWER COMPANY.

FOR PREFABRICATED PEDESTRIAN BRIDGE, SEE SPECIAL PROVISIONS.

FOR BRIDGE APPROACH BOARDWALKS, SEE SPECIAL PROVISIONS.

WATER USED IN THE PRODUCTION OF CONCRETE OR GROUT SHALL BE IN ACCORDANCE WITH SECTION 1024-4 OF THE STANDARD SPECIFICATIONS.

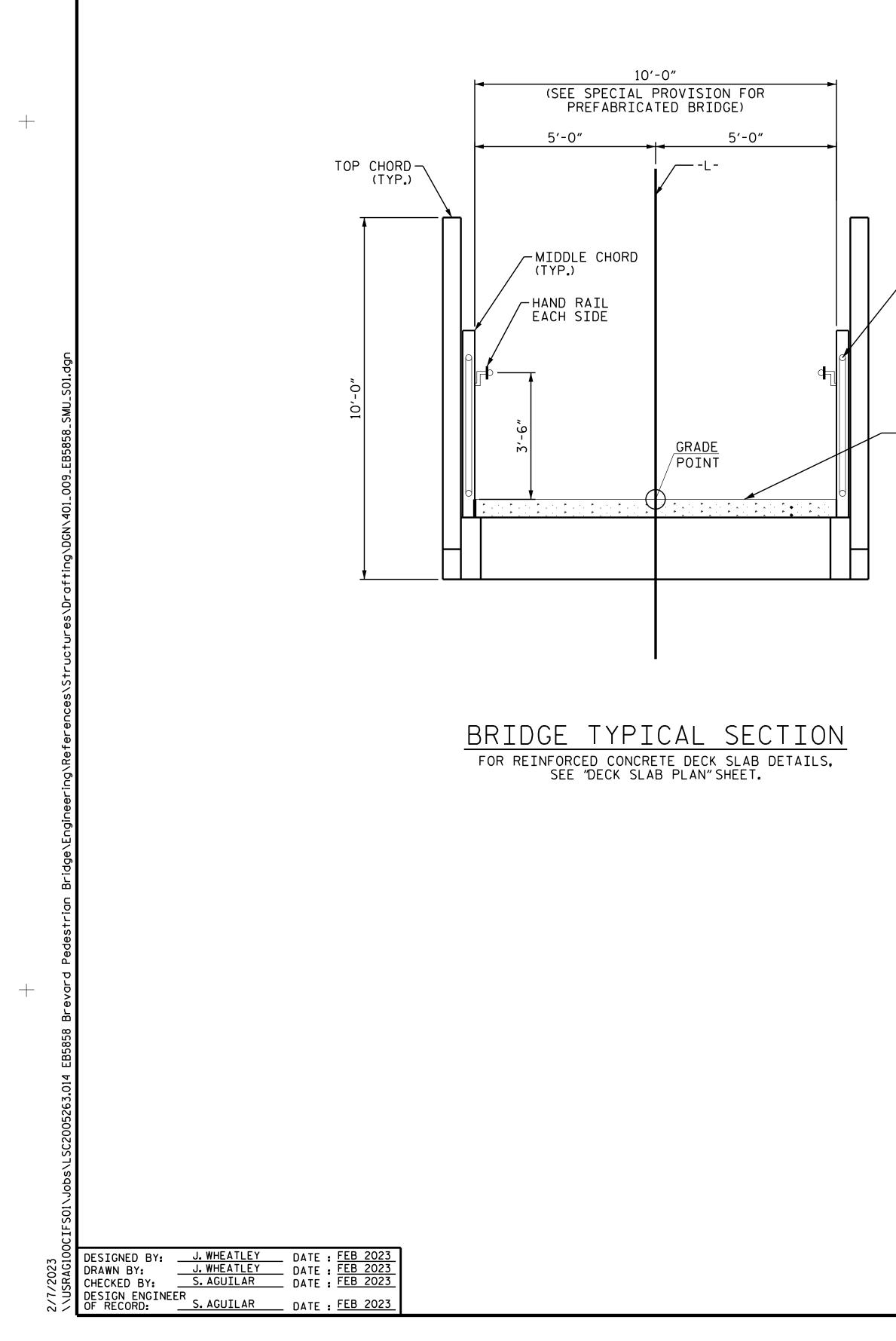


NOTES

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

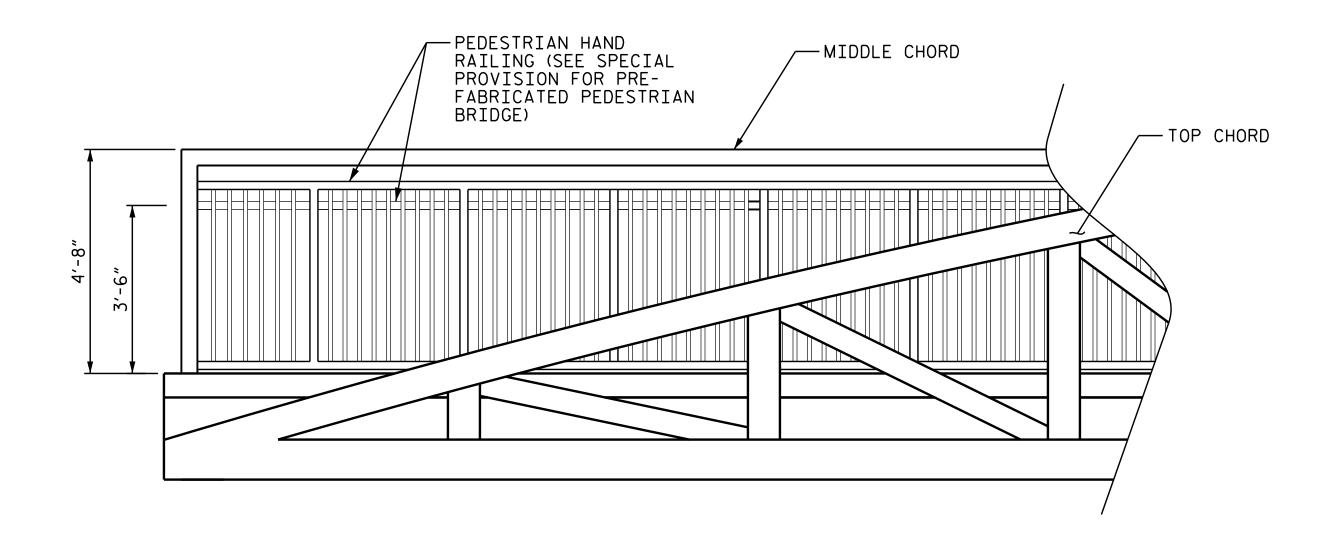
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FENCE SHOULD HAVE OPENINGS NO LARGER THAN 4.0"(TYP.) (SEE SPECIAL PROVISION FOR PRE-FABRICATED PEDESTRIAN BRIDGE.)

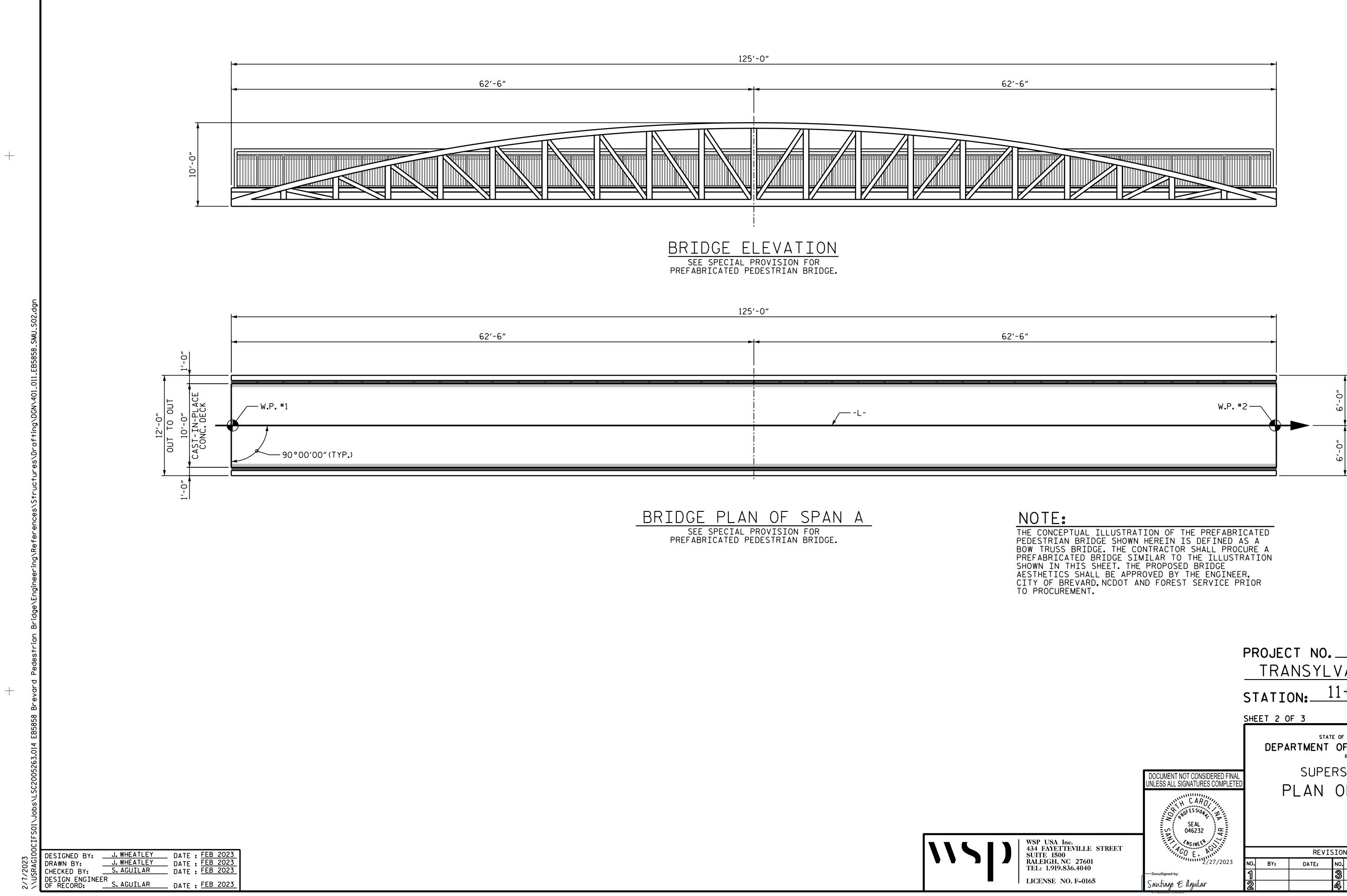
— 5″CAST-IN-PLACE REINFORCED CONCRETE DECK



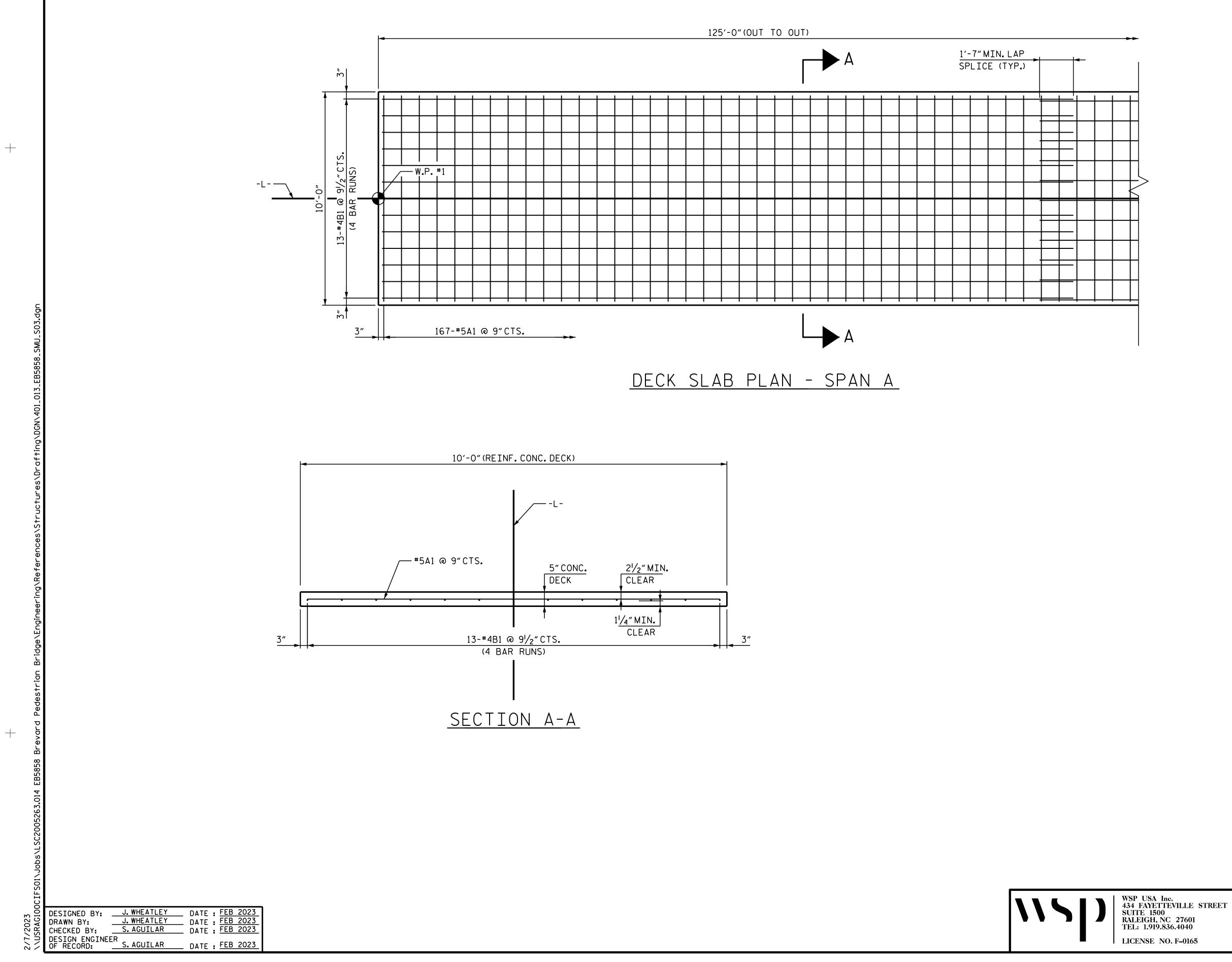


BRIDGE ELEVATION

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	PROJECT NO. <u>EB-5858</u> <u>TRANSYLVANIA</u> COUNTY STATION: <u>11+64.04</u> -L-
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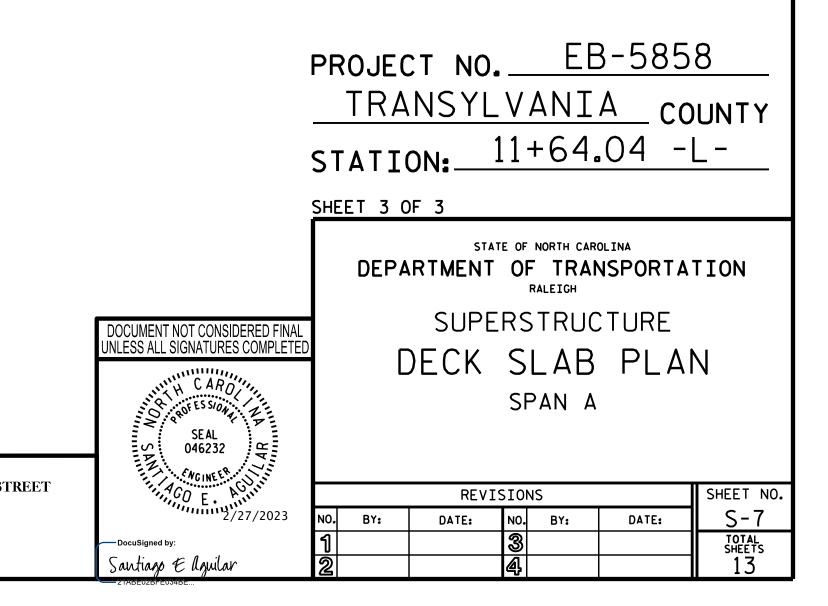


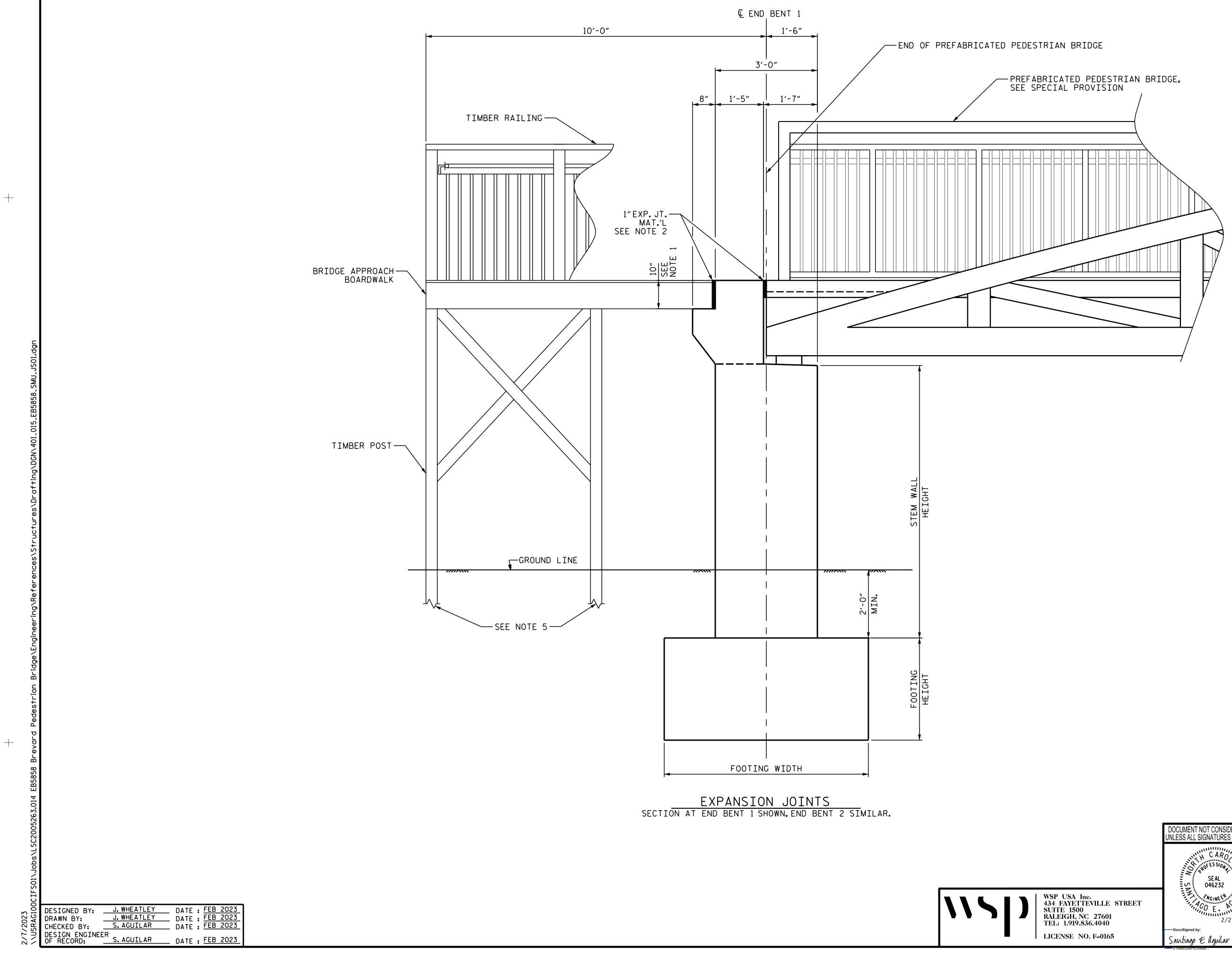
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BILL OF MATERIAL									
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
* A1	167	# 5	STR	9′-8″	1684				
* B1	52	2 # 4 STR 32'-5" 1126							
	EPOXY COATED REINFORCING STEEL 2,810 LBS.								
TOTAL CLASS AA CONCRETE 19.3 C.Y.									

NOTE:

THE CONTRACTOR MAY PROPOSE AN ALTERNATE DESIGN FOR THE DECK SLAB AT NO ADDITIONAL COST TO THE OWNER FOLLOWING THE TECHNICAL RECOMMENDATIONS OF THE PREFABRICATED BRIDGE MANUFACTURER. SUCH DESIGN SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL ALONG WITH THE PREFABRICATED BRIDGE SHOP DRAWINGS. THE ALTERNATE DECK SLAB DESIGN SHALL BE STAMPED BY A PROFESSIONAL ENGINEER IN THE STATE OF NORTH CAROLINA.





NOTE:

- 1. HEIGHT OF BRIDGE APPROACH BOARDWALK SEAT MAY BE ADJUSTED TO FIT THE APPROACH BOARDWALK FLOOR GEOMETRY. ADJUSTMENTS SHALL BE SUCH THAT THE TRANSITION FROM BRIDGE APPROACH BOARDWALKS TO THE PREFABRICATED BRIDGE IS SMOOTH AND FREE OF TRIP HAZARDS. THE BRIDGE APPROACH BOARDWALK CONTRACTOR SHALL SUBMIT SHOP DRAWINGS INCLUDING ANY REQUIRED ADJUSTMENTS FOR APPROVAL BY THE ENGINEER.
- 2. THE EXPANSION JOINT MAY BE MODIFIED BY THE CONTRACTOR TO MEET THE DESIGN REQUIREMENTS OF THE PREFABRICATED PEDESTRIAN BRIDGE AND THE BRIDGE APPROACH BOARDWALK. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL BY THE ENGINEER.
- 3. END BENT SHALL BE CONSTRUCTED BEFORE THE APPROACH BOARDWALK.
- 4. FOR BRIDGE APPROACH BOARDWALKS, SEE SPECIAL PROVISIONS.
- 5. FOR FOUNDATIONS, SEE BRIDGE APPROACH BOARDWALK SPECIAL PROVISIONS.

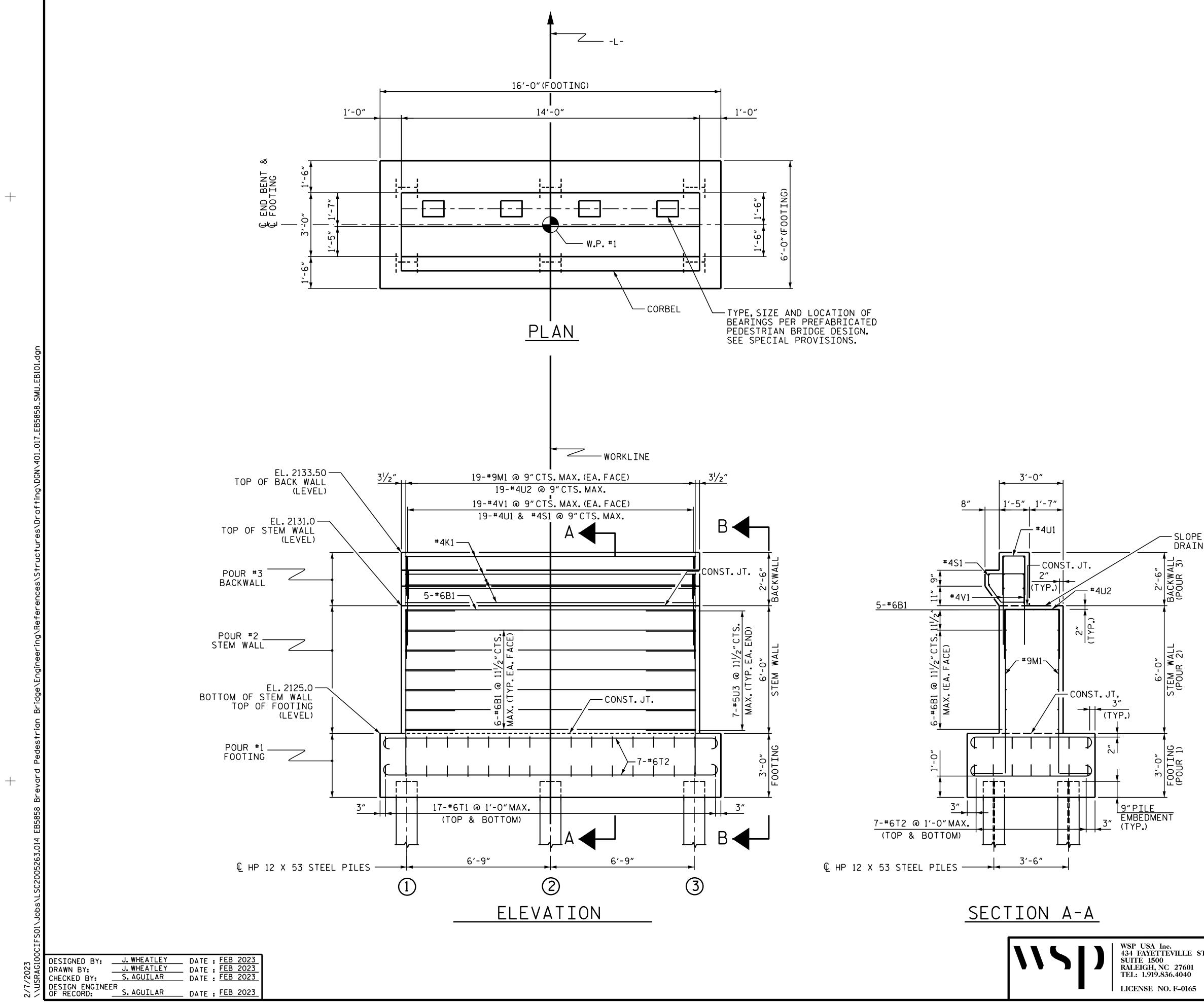
PROJECT NO. EB-5858 TRANSYLVANIA COUNTY STATION: 11+64.04 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

EXPANSION JOINTS

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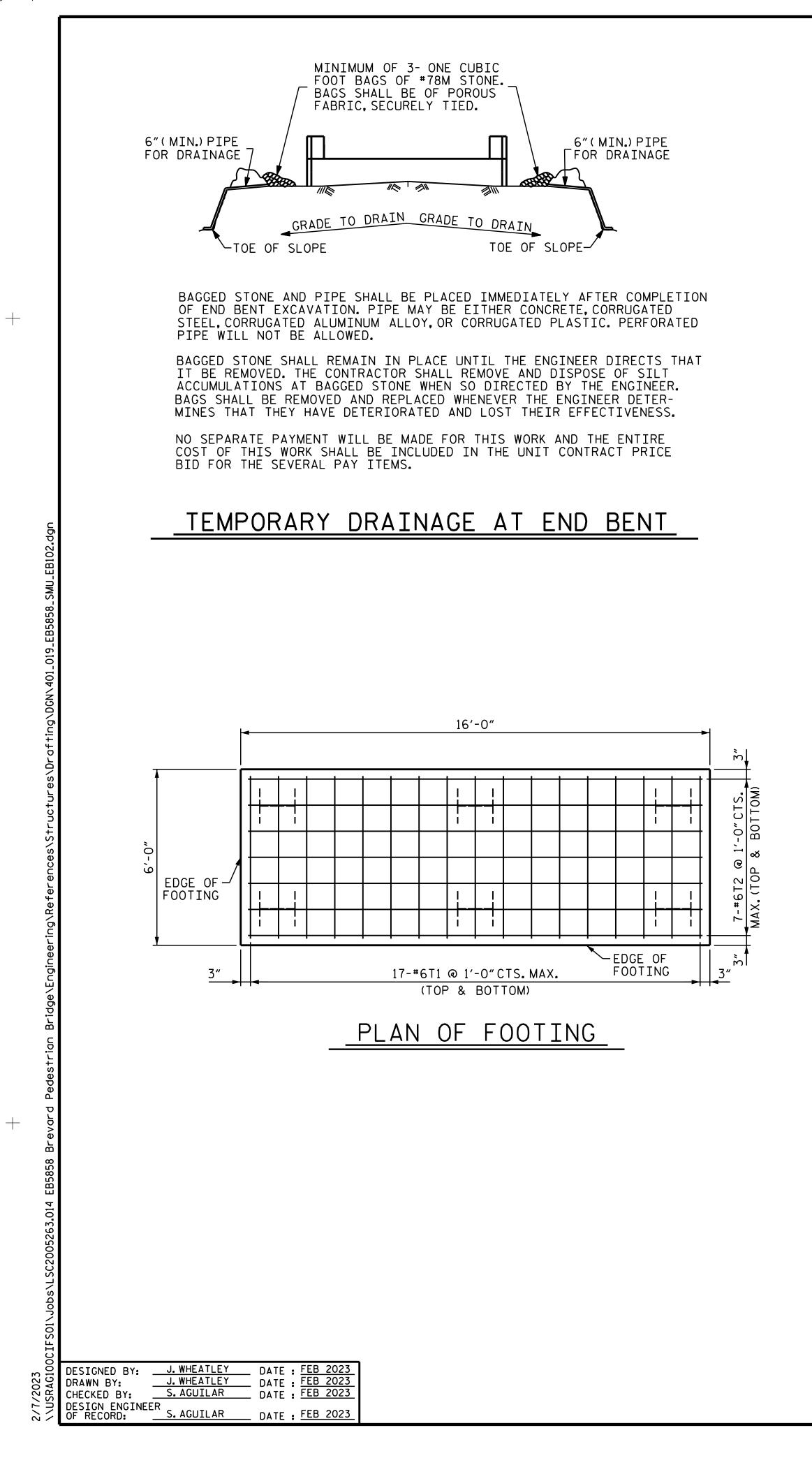


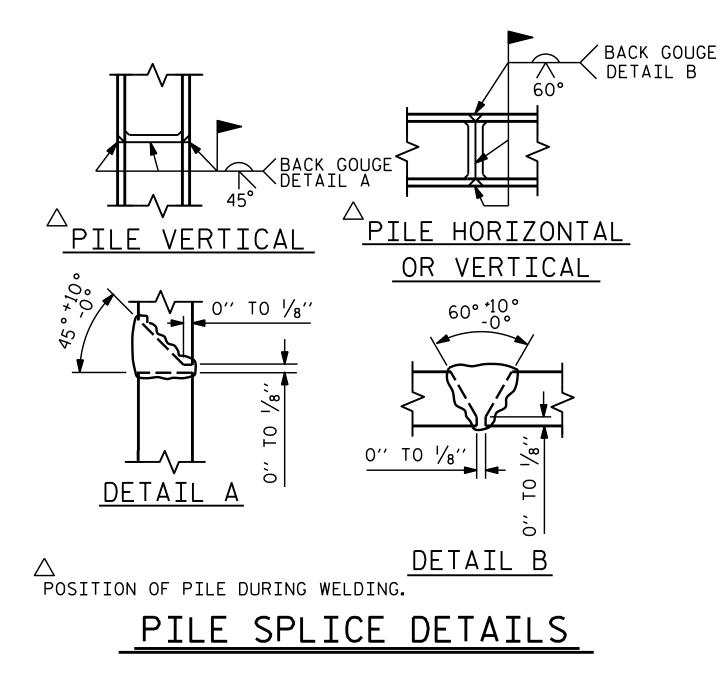
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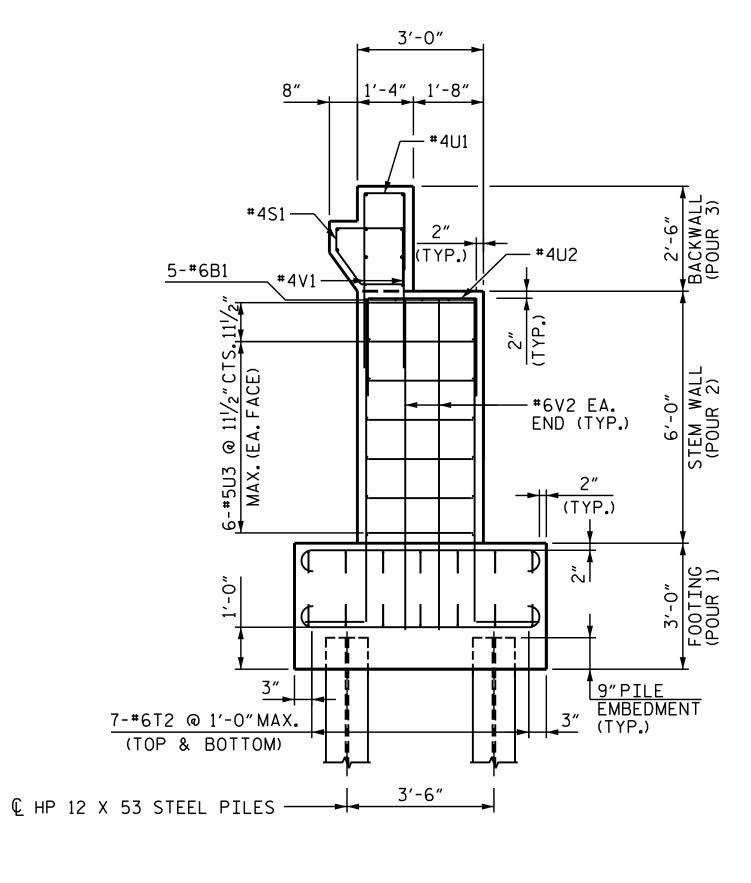
FOR PLAN OF FOOTING, SEE SHEET 2 OF 2. FOR SECTION B-B, SEE SHEET 2 OF 2. FOR PILE SPLICE DETAILS, SEE SHEET 2 OF 2.

-SLOPE FOR DRAINAGE

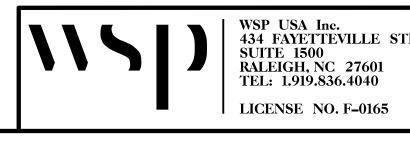
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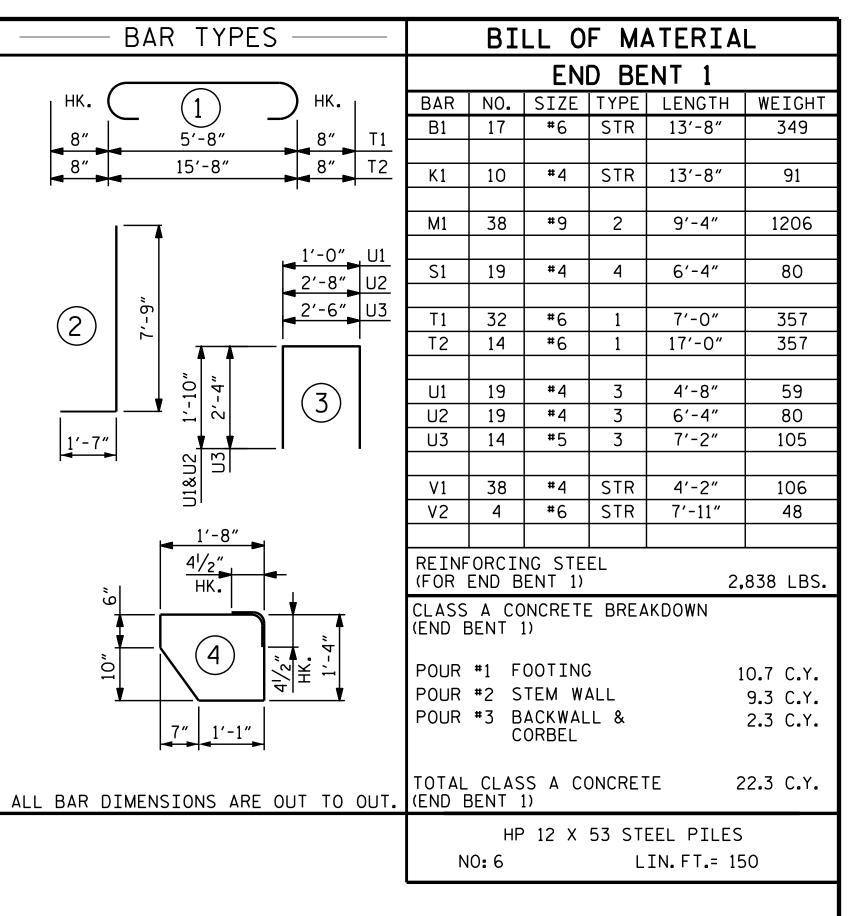




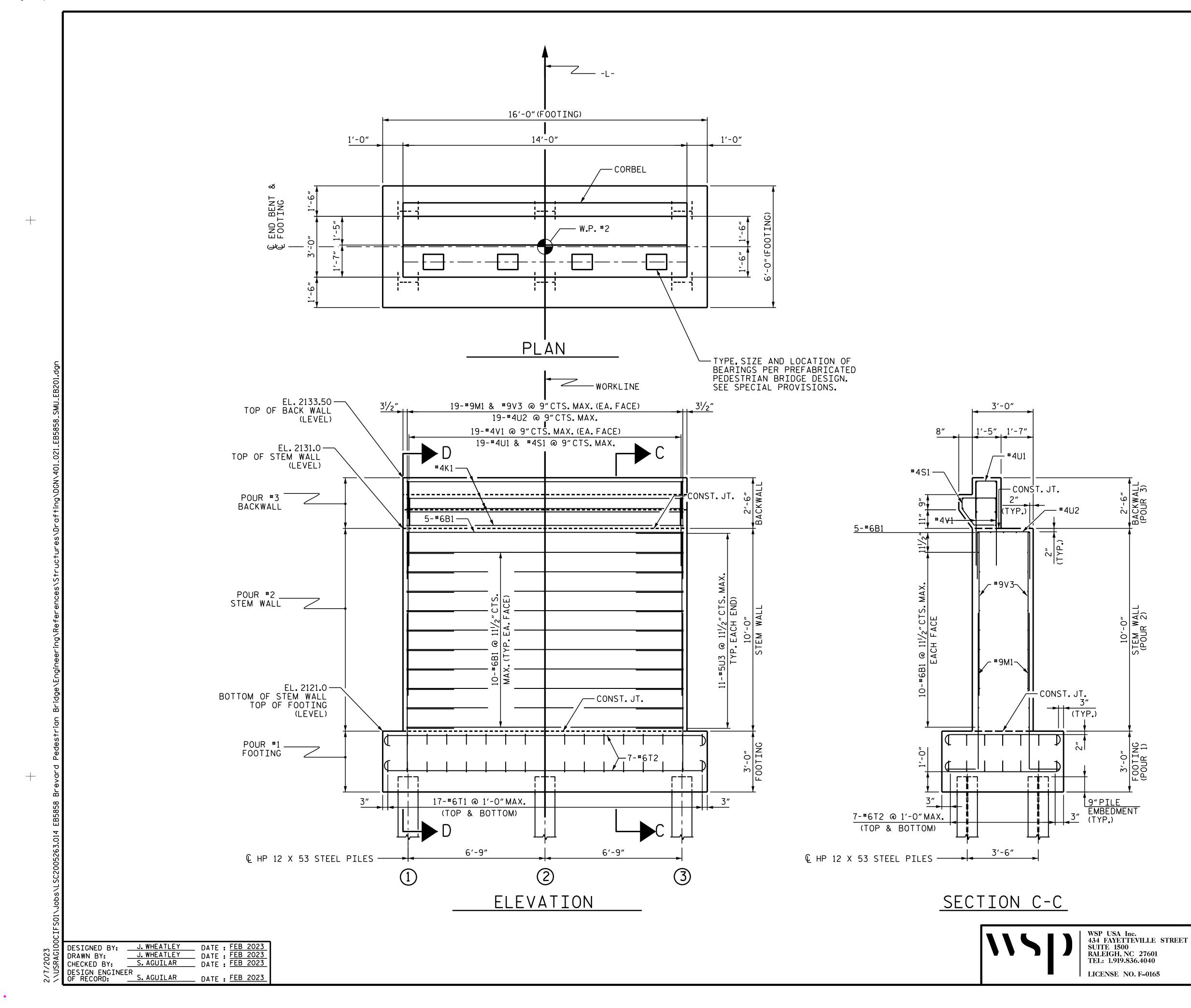


VIEW B-B





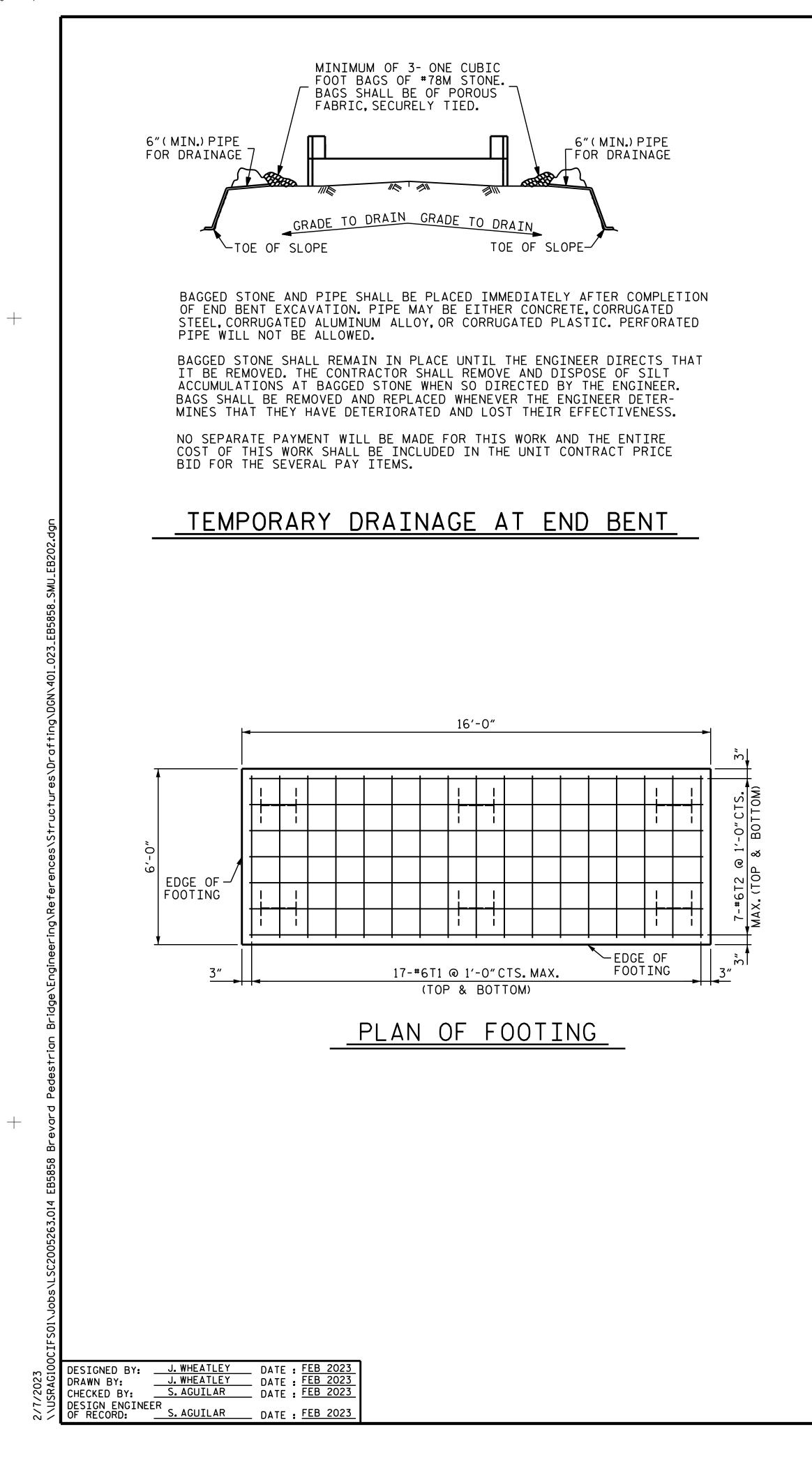
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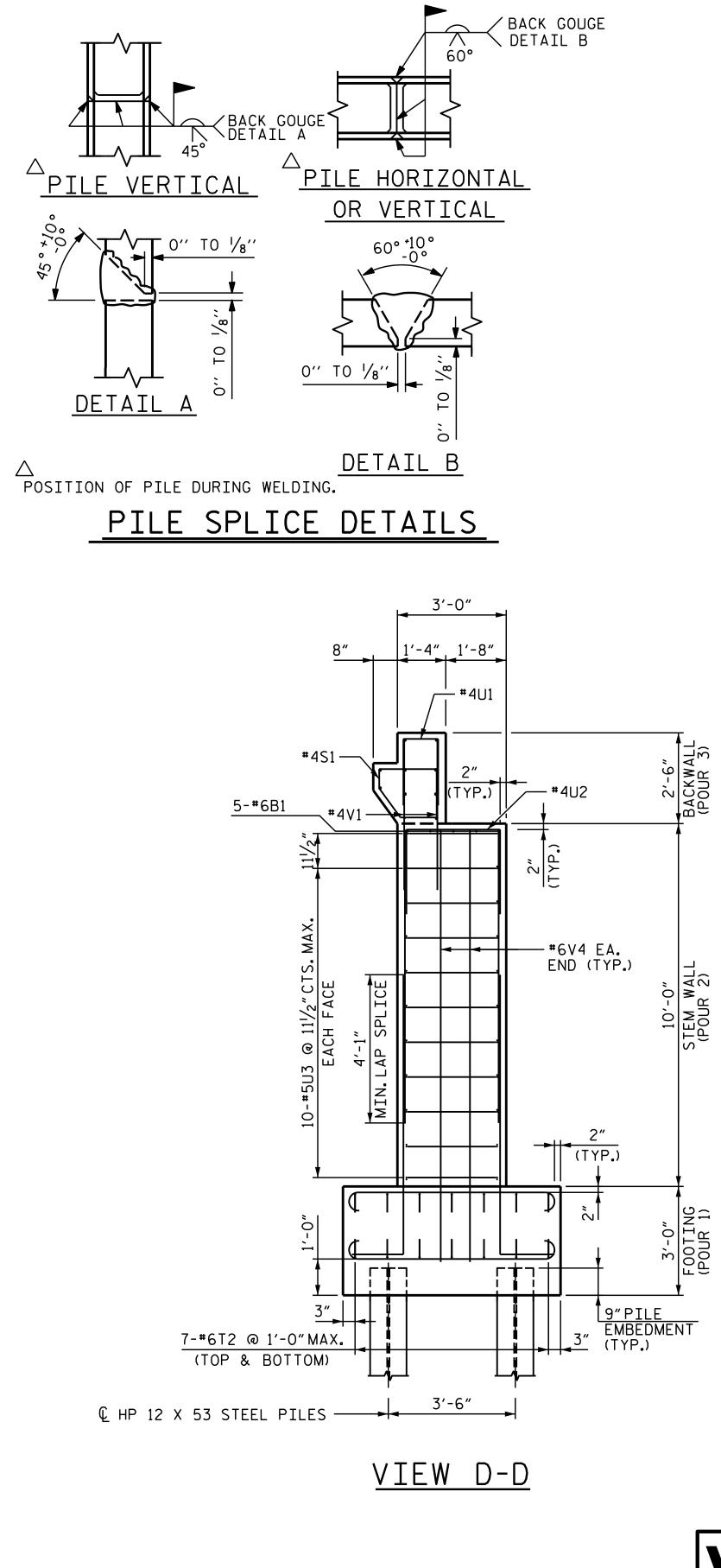


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FOR PLAN OF FOOTING, SEE SHEET 2 OF 2. FOR SECTION B-B, SEE SHEET 2 OF 2. FOR PILE SPLICE DETAILS, SEE SHEET 2 OF 2.

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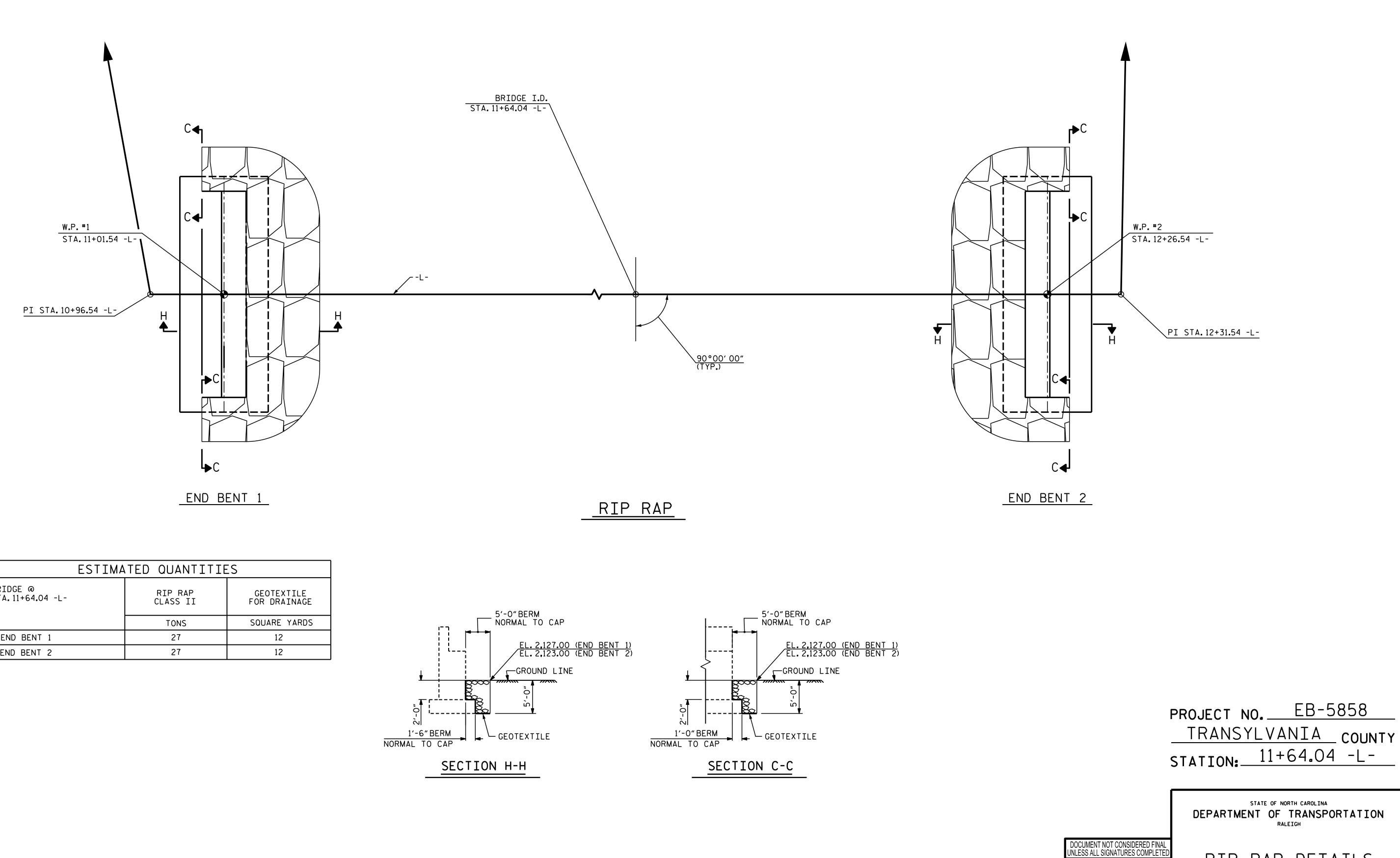


WSP USA Inc. 434 FAYETTEVILLE STR SUITE 1500 RALEIGH, NC 27601 TEL: 1.919.836.4040 LICENSE NO. F-0165

BAR TYPES		BI	LL O	F MA	ATERIA	L
			ΕΝί) BE	NT 2	
HK ((1)) HK	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
8" 5'-8" 8" T1	B1	25	# 6	STR	13'-8"	513
8″ 15′-8″ 8″ T2						
	К1	10	#4	STR	13'-8"	91
		38	#9	2	9'-4"	1200
	M1	00	9	2	5 - 4	1206
	S1	19	#4	4	6'-4"	80
<u>2'-8" U2</u>						
2'-6" U3	T1	34	#6	1	7′-0″	357
	T2	14	# 6	1	17'-0"	357
	U1	19	#4	3	4'-8"	59
	U2	19	#4	3	6'-4"	80
	U3	22	#5	3	7'-2"	164
	<u>\/1</u>	38	#4	STR	4'-2"	106
1	V1 V3	38	#9	STR	8'-1"	106 1044
1'-8"	V4	4	#6	STR	11'-11"	72
41/2"						
	REINF	ORCIN	NG STE	EL		
			ENT 2)		4	129 LBS.
			NCRETE	E BREA	KDOWN	
	(END E	BENT :	2)			
	POUR	#1 F	OOTINC	7	1	0.7 C.Y.
			TEM W			5.6 C.Y.
		#3 B	ACKWAL			2.3 C.Y.
		С	ORBEL			
					· - · · · ·	
ALL BAR DIMENSIONS ARE OUT TO OUT.			S A C(2)			8.6 C.Y.
HP 12 X 53 STEEL PILES						
NO:6 LIN.FT.= 120						
	L					

PROJECT NO. <u>EB-5858</u> <u>TRANSYLVANIA</u> COUNTY STATION: <u>11+64.04</u> -L- SHEET 2 OF 2						
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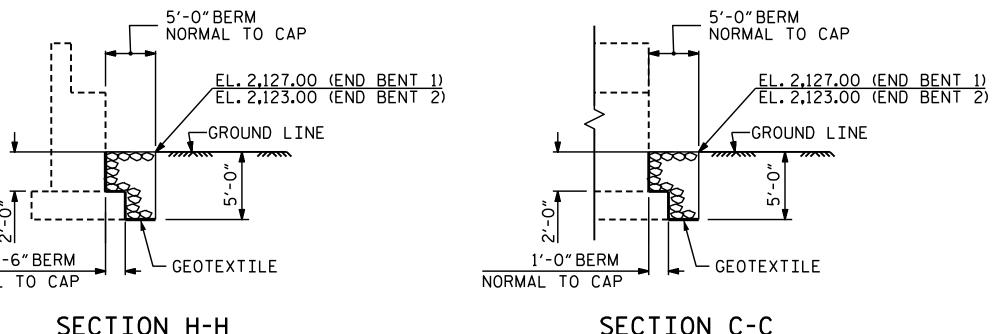
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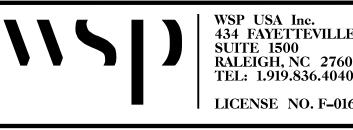
ESTIMATED QUANTITIES						
BRIDGE @ STA.11+64.04 -L-	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS				
END BENT 1	27	12				
END BENT 2	27	12				

DESIGNED BY: J. WHEATLEY DATE : FEB 2023 DRAWN BY: J. WHEATLEY DATE : FEB 2023 CHECKED BY: S. AGUILAR DATE : FEB 2023 DESIGN ENGINEER OF RECORD: S. AGUILAR DATE : FEB 2023

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RIP RAP DETAILS

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601 60	
165	5

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SO.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

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STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE ¾ ″Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - ⅛ ″Ø STUDS FOR 4 - ¾ ″Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF ⅛ ″Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY. AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{1}{16}$ IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY VIGINCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS. BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

HANDRAILS AND POSTS:



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